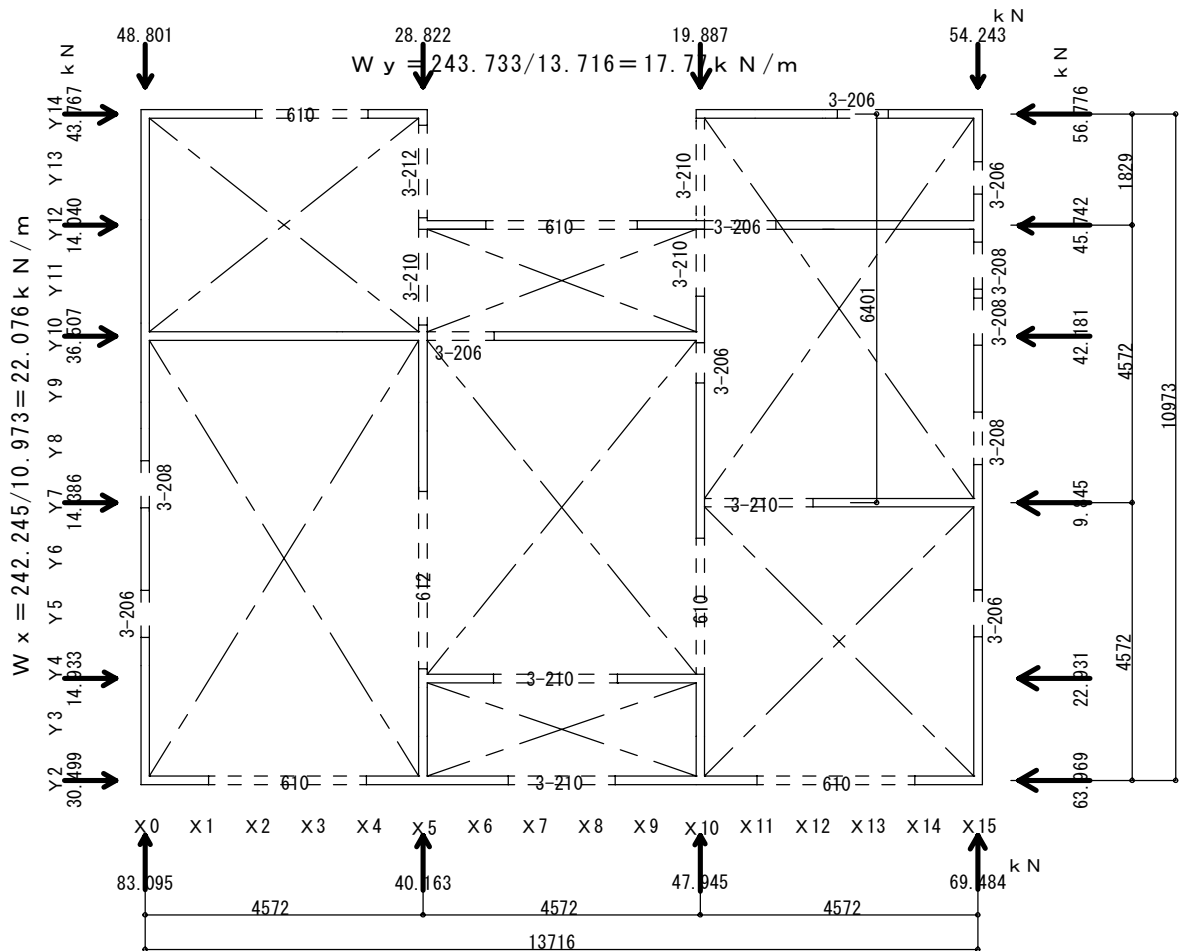


2階頭つなぎ・上枠の設計(耐震等級3にて検討)

$$Q E_x = 242.245 \text{ kN} \quad Q w_{2x} = 103.086 \text{ kN}$$

$$Q E_y = 243.733 \text{ kN} \quad Q w_{2y} = 128.304 \text{ kN}$$

頭つなぎ・上枠 S.P.F 2級 206



① X方向

$$M = 1/8 \times 22.076 \times 6.401^2 = 113.064 \text{ kN} \cdot \text{m}$$

$$T = C = 113064/4.572 = 24729.7 \text{ N}$$

$$f_t = F \times 2/3 = 11.4 \times 2/3 = 7.6 \text{ N/m}^2$$

$$f_c = F \times 2/3 = 17.4 \times 2/3 = 11.6 \text{ N/m}^2$$

$$T < T_{\text{cap}} = 760 \times 0.84 \times 53.2 = 33962.8 \text{ N} > T = 24729.7 \text{ N}$$

○ K

$$C < C_{\text{cap}} = 1160 \times 0.96 \times 53.2 = 59243.5 \text{ N} > C = 24729.7 \text{ N}$$

○ K

② Y方向

$$M = 1/8 \times 17.770 \times 4.572^2 = 46.431 \text{ kN} \cdot \text{m}$$

$$T = C = 46431/10.973 = 4231.4 \text{ N}$$

$$T < T_{\text{cap}} = 33962.8 \text{ N} > T = 4231.4 \text{ N}$$

○ K

$$C < C_{\text{cap}} = 59243.5 \text{ N} > C = 4231.4 \text{ N}$$

○ K